

## 27.1 GENERAL INFORMATION

PERFORMANCE RELATED	MATERIAL	INSTALLATION RELATED

### Product

A high security, high performance, through fixing, torque controlled expansion anchor which has approval for use in cracked and non-cracked concrete.

### Benefits, Advantages and Features

#### European Technical Approval (option1) – ETA-10/0276:

- Tested for Category 1 seismic performance in accordance with ETAG001 Annex E ( Category 2 pending )
- CISMA Report Anchors exposed to seismic actions - NTC 022
- Highest level of European approval for mechanical expansion anchors
- Approved for all directions (floor, wall, overhead)
- Shallow embedment depths
- Highest performance in cracked concrete
- Zinc Plated to 5µm
- Anchor diameters from M6 to M20

#### Suitable for structural loads:

- Safety critical loads
- High tensile capacity of Grade 8.8 Steel Bolt.
- Heavy duty, heat treated washer. Heavy duty, thick expansion sleeve that provides secure grip to concrete.

#### Improved security:

- Large expansion reserve that ensures retention in concrete if overloaded.
- Torque induced pull down closes gaps and induces preload.

#### Resistant to cyclic loading:

- Heavy duty sleeve with integrated pull-down section works to retain 65% of initial preload.

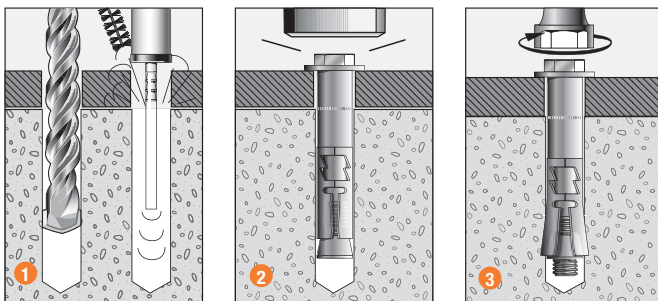
#### Fast installation:

- Hex Nut & Hex Bolt versions available
- Countersunk heads available.
- Through fixing eliminates marking out and repositioning of fixtures.

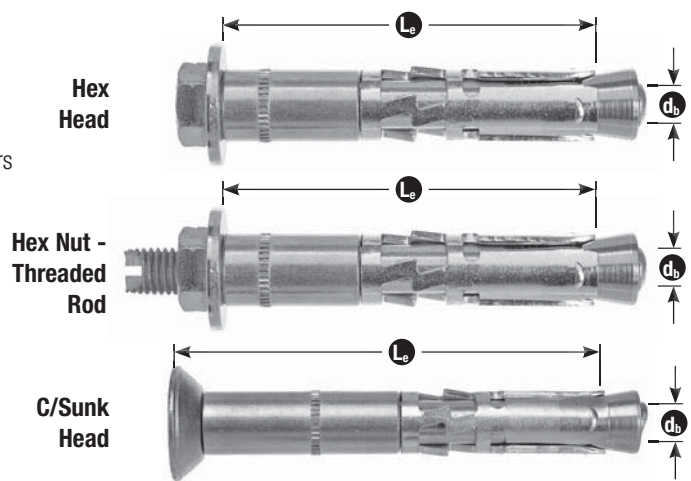
**Fire rated: Refer Fire rated mechanical anchor section.**

**Cracked Concrete: Refer to Cracked Concrete section.**

### Installation



1. Drill or core a hole to the recommended diameter and depth using the fixture as a template. Clean the hole thoroughly with a hole cleaning brush.  
Remove the debris with a hand pump, compressed air, or vacuum.
2. After ensuring that the anchor is assembled correctly, insert the anchor through the fixture and drive with a hammer until the washer contacts the fixture.
3. Tighten the bolt with a torque wrench to the specified assembly torque.



## Principal Applications

- Structural beams and columns
- Anchoring braces for precast panels
- Safety barriers
- Racking
- Machinery and heavy plant hold down
- Lift guide rails
- Commercial building facades

### Installation and performance details

Anchor Size, d <sub>b</sub> (mm)	Installation details				Optimum dimensions*		Reduced Characteristic Capacity #			
	Drilled Hole diam., d <sub>h</sub> (mm)	Fixture hole diameter, d <sub>f</sub> (mm)	Anchor effective depth, h (mm)	Tightening torque, T <sub>r</sub> (Nm)	Edge* distance, e <sub>c</sub> (mm)	Anchor spacing, a <sub>c</sub> (mm)	Steel	Concrete		
							Shear, ØV <sub>us</sub> (kN)	Tension, ØN <sub>uc</sub> (kN)**		
								Concrete compressive strength, f' <sub>c</sub>		
20 MPa	32 MPa	40 MPa								
M10	15	17	60	50	100	180	23.0	15.5	19.5	21.8
			70		105	210	38.5	19.5	24.6	27.5
			80		120	240	38.5	23.8	30.1	33.6
M12	18	20	70	80	120	210	33.5	19.5	24.6	27.5
			85		130	255	55.1	26.1	33.0	36.8
			95		145	285	55.1	30.8	38.9	43.5
M16	24	26	95	120	160	285	62.3	30.8	38.9	43.5
			105		160	315	104.5	35.8	45.2	50.6
			115		175	345	104.5	41.0	51.9	58.0
M20	28	32	110	200	205	330	100.9	38.4	48.5	54.2
			130		205	375	151.7	49.3	62.3	69.7
			140		210	420	151.7	55.1	69.7	77.9

\* Note: For shear loads acting towards an edge or where these optimum dimensions are not achievable, please use the simplified strength limit state design process to verify capacity.

\*\*Note: Reduced characteristic ultimate concrete tensile capacity = ØN<sub>uc</sub> where Ø = 0.67 and N<sub>uc</sub> = Characteristic ultimate concrete tensile capacity. For conversion to Working Load Limit MULTIPLY ØN<sub>uc</sub> x 0.50

## 27.2 DESCRIPTION AND PART NUMBERS

Anchor size, d <sub>b</sub>	Drilled hole diameter, d <sub>h</sub> (mm)	Effective length, L <sub>e</sub> (mm)	Part Number		
			Zinc (Hex Hd)	Stainless Steel (Hex Hd)	Zinc (C/Sunk Hd)
M10	15	90	SP10105	-	-
		97	-	-	SP10105F
		95	-	SP10110SS**	-
M12	18	90	SP12105	-	-
		105	SP12120	SP12120SS**	-
		120	-	-	SP12120F**
M16	24	120	-	SP16145ESS* **	-
		125	SP16145	-	-
M20	28	150	SP20170	-	-

Effective depth, h (mm)

$$h = L_e - t$$

t = total thickness of material(s) being fixed

Substrate thickness, b<sub>m</sub> (mm)

$$b_m = h + (3.5 \times d_h)$$

Drilled hole depth, h<sub>1</sub> (mm)

$$h_1 = h + (1.5 \times d_h)$$

h = Effective depth

\*Note hex nut assembly. \*\*Note: Lead times apply.

Note: For more sizes and different head styles, refer to page 242, 284 or 304.

## 27.3 ENGINEERING PROPERTIES - Carbon Steel

Anchor size, d <sub>b</sub>	Shank diameter, d <sub>s</sub> (mm)	Bolt stress area, A <sub>s</sub> (mm <sup>2</sup> )	Bolt yield strength, f <sub>y</sub> (MPa)	Bolt UTS, f <sub>u</sub> (MPa)	Spacer area, A <sub>s</sub> (mm <sup>2</sup> )	Spacer yield strength, f <sub>y</sub> (MPa)	Spacer UTS, f <sub>u</sub> (MPa)	Section modulus Z (mm <sup>3</sup> )
M10	9.8	58.0	640	800	83.4	350	480	62.3
M12	11.7	84.3	640	800	119.8	330	430	109.2
M16	15.7	157.0	640	800	201.7	330	430	277.5
M20	19.7	245.0	660	830	242.5	330	430	540.9

## ENGINEERING PROPERTIES - 316 Stainless Steel

Anchor size, d <sub>b</sub>	Shank diameter, d <sub>s</sub> (mm)	Bolt stress area, A <sub>s</sub> (mm <sup>2</sup> )	Bolt yield strength, f <sub>y</sub> (MPa)	Bolt UTS, f <sub>u</sub> (MPa)	Spacer area, A <sub>s</sub> (mm <sup>2</sup> )	Spacer yield strength, f <sub>y</sub> (MPa)	Spacer UTS, f <sub>u</sub> (MPa)	Section modulus Z (mm <sup>3</sup> )
M10	9.8	58.0	600	800	65.2	260	585	62.3
M12	11.7	84.3	640	800	101.6	260	585	109.2
M16	15.7	157.0	350	700	198.0	260	585	277.5

**STEP 1** Select anchor to be evaluated

Table 1a Indicative combined loading – interaction diagram

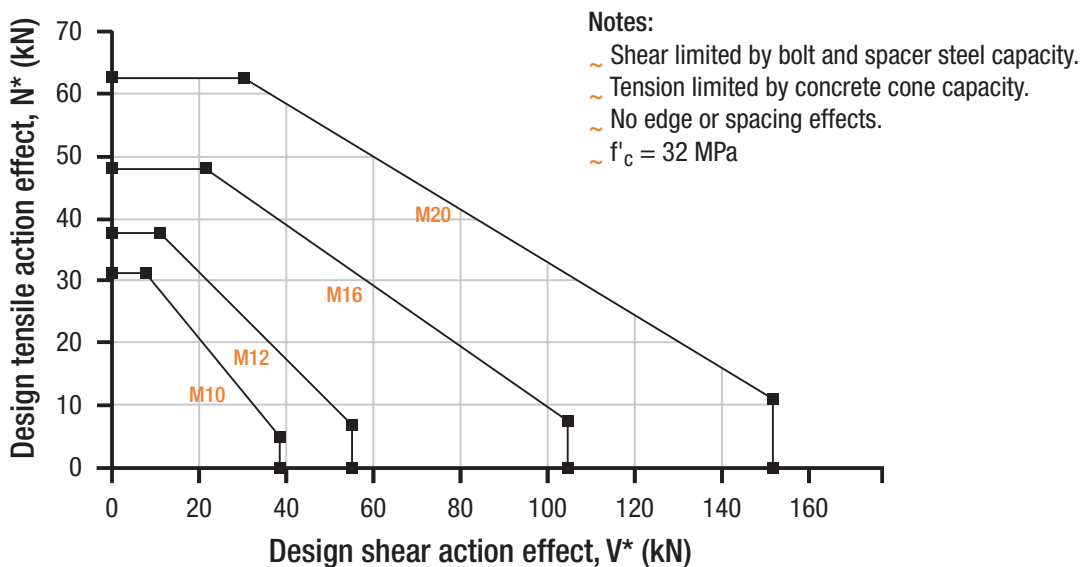


Table 1b Absolute minimum edge distance and anchor spacing values,  $e_m$  and  $a_m$  (mm)

Anchor size, $d_b$	M10	M12	M16	M20
Edge distance, $e_m$	100	120	160	205
Anchor spacing, $a_m$	65	80	105	135

**Step 1c Calculate anchor effective depth, h (mm)**

Refer to "Description and Part Numbers" table on page 186.

Effective depth, h (mm)

$$h = L_e - t$$

t = total thickness of material(s) being fixed

**Checkpoint**

**1** Anchor size determined, absolute minimum compliance achieved, effective depth (h) calculated.

## STEP 2 Verify concrete tensile capacity - per anchor

Table 2a Reduced characteristic ultimate concrete tensile capacity,  $\phi N_{uc}$  (kN),  $\phi_c = 0.67$ ,  $f'_c = 32$  MPa

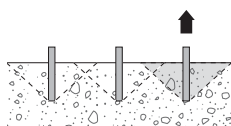
Anchor size, $d_b$	M10	M12	M16	M20
Drilled hole dia., $d_h$ (mm)	15	18	24	28
Effective depth, $h$ (mm)				
60	19.5			
65	22.0			
70	24.6	24.6		
75	27.3	27.3		
80	<b>30.1</b>	30.1		
85	33.0	33.0		
90		35.9		
95		<b>38.9</b>	38.9	
100			42.0	
105			45.2	
110			<b>48.5</b>	48.5
115			51.9	51.9
120			55.3	55.3
130			62.3	<b>62.3</b>
140				69.7

Note: Effective depth,  $h$  must be  $\geq 4 \times$  drilled hole diameter,  $d_h$  for anchor to achieve tabled shear capacities.

Table 2b Concrete compressive strength effect, tension,  $X_{nc}$

$f'_c$ (MPa)	20	25	32	40	50	60
$X_{nc}$	0.79	0.88	1.00	1.12	1.25	1.37

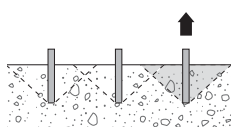
Table 2c Edge distance effect, tension,  $X_{ne}$



Edge distance, $e$ (mm)	100	125	150	175	200	250
Effective depth, $h$ (mm)						
60	1.00					
65	1.00					
70	0.97	1.00				
75	0.92	1.00				
80	0.88	1.00	1.00			
85	0.85	0.99	1.00	1.00		
90	0.82	0.95	1.00	1.00		
95	0.79	0.91	1.00	1.00	1.00	
100	0.77	0.88	1.00	1.00	1.00	
110	0.72	0.83	0.94	1.00	1.00	1.00
120	0.69	0.79	0.88	0.98	1.00	1.00
130	0.66	0.75	0.84	0.93	1.00	1.00
140	0.63	0.72	0.80	0.88	0.97	1.00

Table 2d Anchor spacing effect, end of a row, tension,  $X_{nae}$

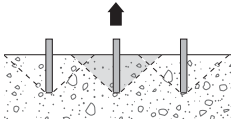
Note: For single anchor designs,  $X_{nae} = 1.0$



Anchor spacing, $a$ (mm)	75	100	125	150	175	200	250	300	350	400
Effective depth, $h$ (mm)										
60	0.71	0.78	0.85	0.92	0.99					
65	0.69	0.76	0.82	0.88	0.95	1.00				
70	0.68	0.74	0.80	0.86	0.92	0.98				
75	0.67	0.72	0.78	0.83	0.89	0.94				
80	0.66	0.71	0.76	0.81	0.86	0.92	1.00			
85	0.65	0.70	0.75	0.79	0.84	0.89	0.99			
90	0.64	0.69	0.73	0.78	0.82	0.87	0.96			
95	0.63	0.68	0.72	0.76	0.81	0.85	0.94			
100	0.63	0.67	0.71	0.75	0.79	0.83	0.92	1.00		
110	0.61	0.65	0.69	0.73	0.77	0.80	0.88	0.95		
120	0.60	0.64	0.67	0.71	0.74	0.78	0.85	0.92	1.00	
130	0.60	0.63	0.66	0.69	0.72	0.76	0.82	0.88	0.95	1.00
140	0.59	0.62	0.65	0.68	0.71	0.74	0.80	0.86	0.92	0.98

Table 2e Anchor spacing effect, internal to a row, tension,  $X_{nai}$

Note: For single anchor designs,  $X_{nai} = 1.0$



Anchor spacing, a (mm)	75	100	125	150	175	200	250	300	350	400
Effective depth, h (mm)										
60	0.42	0.56	0.69	0.83	0.97					
65	0.38	0.51	0.64	0.77	0.90					
70	0.36	0.48	0.60	0.71	0.83	0.95				
75	0.33	0.44	0.56	0.67	0.78	0.89				
80	0.31	0.42	0.52	0.63	0.73	0.83				
85	0.29	0.39	0.49	0.59	0.69	0.78	0.98			
90	0.28	0.37	0.46	0.56	0.65	0.74	0.93			
95	0.26	0.35	0.44	0.53	0.61	0.70	0.88			
100	0.25	0.33	0.42	0.50	0.58	0.67	0.83	1.00		
110	0.23	0.30	0.38	0.45	0.53	0.61	0.76	0.91	1.00	
120	0.21	0.28	0.35	0.42	0.49	0.56	0.69	0.83	0.97	
130	0.19	0.26	0.32	0.38	0.45	0.51	0.64	0.77	0.90	1.00
140	0.18	0.24	0.30	0.36	0.42	0.48	0.60	0.71	0.83	0.95

**Checkpoint 2**

Design reduced ultimate concrete tensile capacity,  $\phi N_{urc}$

$$\phi N_{urc} = \phi N_{uc} * X_{nc} * X_{ne} * (X_{nae} \text{ or } X_{nai})$$

**STEP 3**

Verify anchor tensile capacity - per anchor

Table 3a Reduced characteristic ultimate steel tensile capacity,  $\phi N_{us}$  (kN),  $\phi_n = 0.8$

Anchor size, $d_b$	M10	M12	M16	M20
Carbon steel	37.1	54.0	100.5	162.7
316 Stainless Steel	37.1	54.0	87.9	

Step 3b Reduced characteristic ultimate bolt steel tensile capacity,  $\phi N_{tf}$  (kN)

Not appropriate for this product.

**Checkpoint 3**

Design reduced ultimate tensile capacity,  $\phi N_{ur}$

$$\phi N_{ur} = \text{minimum of } \phi N_{urc}, \phi N_{us}$$

Check  $N^* / \phi N_{ur} \leq 1$ ,

if not satisfied return to step 1

Tensile performance conversion table

Performance Required	Concrete Tensile Performance		Steel Tensile Performance	
	Notation	Concrete Tension Capacity	Notation	Carbon Steel Tension Capacity
Strength Limit State	$\phi N_{urc}$	MULTIPLY $\phi N_{urc} \times 1.00$	$\phi N_{us}$	MULTIPLY $\phi N_{us} \times 1.00$
Working Load Limit	$N_{ac}$	MULTIPLY $\phi N_{urc} \times 0.50$	$N_{as}$	MULTIPLY $\phi N_{us} \times 0.56$
Cyclic Loading	$N_{yc}$	MULTIPLY $\phi N_{urc} \times 0.50$	$N_{ys}$	MULTIPLY $\phi N_{us} \times 0.56$
Fire Resistance	$N_{Rk,c,fi,t}$	Refer to pages 238-257	$N_{Rk,s,fi,t}$	Refer to pages 238-257
Cracked Concrete/Tension Zone	$N_{Rd,p}^0$	Refer to pages 258-298	$N_{Rd,s}$	Refer to pages 258-298
Seismic	$N_{Rd,p,sis}^0$	Refer to pages 299-325	$N_{Rd,s,sis}$	Refer to pages 299-325

NOTE: Design Tensile Capacity is the minimum of Concrete Tension and Steel Tension Capacities

## STEP 4 Verify concrete shear capacity - per anchor

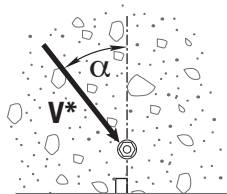
Table 4a Reduced characteristic ultimate concrete edge shear capacity,  $\phi V_{uc}$  (kN),  $\phi_q = 0.6$ ,  $f'_c = 32$  MPa

Anchor size, $d_b$	M10	M12	M16	M20
Edge distance, $e$ (mm)				
100	16			
125	22.4	24.6		
150	29.5	32.3		
175	37.1	40.7	47.0	
200	45.4	49.7	57.4	62
250	63.4	69.4	80.2	86.6
300	83.3	91.3	105.4	113.9
400	128.3	140.5	162.3	175.3
500		196.4	226.8	245.0
600		258.2	298.1	322.0
800			459.0	495.8
1000				692.9

Note: Effective depth,  $h$  must be  $\geq 4 \times$  drilled hole diameter,  $d_h$  for anchor to achieve tabled shear capacities.

Table 4b Concrete compressive strength effect, concrete edge shear,  $X_{vc}$

$f'_c$ (MPa)	20	25	32	40	50	60
$X_{vc}$	0.79	0.88	1.00	1.12	1.25	1.37



Load direction effect, conc. edge shear,  $X_{vd}$

Table 4c Load direction effect, concrete edge shear,  $X_{vd}$

Angle, $\alpha^\circ$	0	10	20	30	40	50	60	70	80	90 - 180
$X_{vd}$	1.00	1.04	1.16	1.32	1.50	1.66	1.80	1.91	1.98	2.00

Table 4d Anchor spacing effect, concrete edge shear,  $X_{va}$

Note: For single anchor designs,  $X_{va} = 1.0$

Edge distance, $e$ (mm)	100	125	150	175	200	250	300	400	500	600	800	1000
Anchor spacing, $a$ (mm)												
75	0.65	0.62	0.60	0.59	0.58	0.56	0.55	0.54				
85	0.67	0.64	0.61	0.60	0.59	0.57	0.56	0.54				
100	0.70	0.66	0.63	0.61	0.60	0.58	0.57	0.55	0.54			
120	0.74	0.69	0.66	0.64	0.62	0.60	0.58	0.56	0.55	0.54		
150	0.80	0.74	0.70	0.67	0.65	0.62	0.60	0.58	0.56	0.55	0.54	
175	0.85	0.78	0.73	0.70	0.68	0.64	0.62	0.59	0.57	0.56	0.54	0.54
200	0.90	0.82	0.77	0.73	0.70	0.66	0.63	0.60	0.58	0.57	0.55	0.54
300	1.00	0.98	0.90	0.84	0.80	0.74	0.70	0.65	0.62	0.60	0.58	0.56
400		1.00	1.00	0.96	0.90	0.82	0.77	0.70	0.66	0.63	0.60	0.58
600				1.00	1.00	0.98	0.90	0.80	0.74	0.70	0.65	0.62
800						1.00	1.00	0.90	0.82	0.77	0.70	0.66
1000								1.00	0.90	0.83	0.75	0.70
1200									0.98	0.90	0.80	0.74
1500									1.00	1.00	0.88	0.80
1800											0.95	0.86
2100											1.00	0.92
2500												1.00

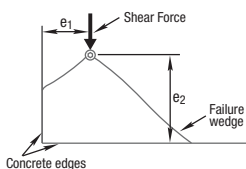
**Table 4e Multiple anchors effect, concrete edge shear,  $X_{vn}$**

Note: For single anchor designs,  $X_{vn} = 1.0$

Anchor spacing / Edge distance, a / e	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00	2.25	2.50
Number of anchors, n												
2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
3	0.72	0.76	0.80	0.83	0.86	0.88	0.91	0.93	0.95	0.96	0.98	1.00
4	0.57	0.64	0.69	0.74	0.79	0.82	0.86	0.89	0.92	0.94	0.97	1.00
5	0.49	0.57	0.63	0.69	0.74	0.79	0.83	0.87	0.90	0.93	0.97	1.00
6	0.43	0.52	0.59	0.66	0.71	0.77	0.81	0.85	0.89	0.93	0.96	1.00
7	0.39	0.48	0.56	0.63	0.69	0.75	0.80	0.84	0.88	0.92	0.96	1.00
8	0.36	0.46	0.54	0.61	0.68	0.74	0.79	0.84	0.88	0.92	0.96	1.00
9	0.34	0.44	0.52	0.60	0.67	0.73	0.78	0.83	0.87	0.91	0.96	1.00
10	0.32	0.42	0.51	0.59	0.66	0.72	0.77	0.82	0.87	0.91	0.96	1.00
15	0.26	0.37	0.47	0.55	0.63	0.70	0.76	0.81	0.86	0.90	0.95	1.00
20	0.23	0.35	0.45	0.54	0.61	0.68	0.75	0.80	0.85	0.90	0.95	1.00

**Table 4f Anchor at a corner effect, concrete edge shear,  $X_{vs}$**

Note: For  $e_1/e_2 > 1.25$ ,  $X_{vs} = 1.0$



ANCHOR AT A CORNER

Edge distance, $e_2$ (mm)	25	30	35	50	60	75	125	200	300	400	600	900
Edge distance, $e_1$ (mm)												
25	0.86	0.77	0.70	0.58	0.53	0.49	0.41	0.37	0.35	0.34	0.32	0.32
30	0.97	0.86	0.78	0.64	0.58	0.52	0.43	0.38	0.36	0.34	0.33	0.32
35	1.00	0.95	0.86	0.69	0.63	0.56	0.46	0.40	0.37	0.35	0.33	0.32
50	1.00	1.00	1.00	0.86	0.77	0.67	0.52	0.44	0.39	0.37	0.35	0.33
60	1.00	1.00	1.00	0.97	0.86	0.75	0.57	0.47	0.41	0.38	0.36	0.34
75	1.00	1.00	1.00	1.00	1.00	0.86	0.64	0.51	0.44	0.41	0.37	0.35
125	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.65	0.53	0.48	0.42	0.38
200	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67	0.58	0.49	0.42
300	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.72	0.58	0.49
400	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67	0.55
500	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.77	0.61
600	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67
900	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86

**Checkpoint 4**

Design reduced ultimate concrete edge shear capacity,  $\phi V_{urc}$

$$\phi V_{urc} = \phi V_{uc} * X_{vc} * X_{vd} * X_{va} * X_{vn} * X_{vs}$$

**STEP 5**

Verify anchor shear capacity - per anchor

**Table 5a Reduced characteristic ultimate steel shear capacity,  $\phi V_{us}$  (kN),  $\phi_v = 0.8$**

Anchor size, $d_b$	M10	M12	M16	M20
<b>Full Shear, (bolt and spacer) (kN)</b>				
Carbon Steel	38.5	55.1	104.5	151.7
316 Stainless Steel	41.9	62.9	112.0	
h min to achieve full shear (mm)	70	80	105	130
<b>Bolt Shear only (kN)</b>				
Carbon Steel	23.0	33.5	62.3	100.9
316 Stainless Steel	23.0	33.5	54.5	
h min to achieve full shear (mm)	60	72	96	112

**Step 5b Reduced characteristic ultimate bolt steel shear capacity,  $\emptyset V_{sf}$  (kN)**

Not appropriate for this product.

**Checkpoint 5**

Design reduced ultimate shear capacity,  $\emptyset V_{ur}$

$\emptyset V_{ur} = \text{minimum of } \emptyset V_{urc}, \emptyset V_{us}$

Check  $V^* / \emptyset V_{ur} \leq 1$ ,

if not satisfied return to step 1

**Shear performance conversion table**

Performance Required	Concrete Shear Performance		Steel Shear Performance	
	Notation	Concrete Shear Capacity	Notation	Carbon Steel Shear Capacity
Strength Limit State	$\emptyset V_{uc}$	MULTIPLY $\emptyset V_{uc}$ x 1.00	$\emptyset V_{us}$	MULTIPLY $\emptyset V_{us}$ x 1.00
Working Load Limit	$V_{ac}$	MULTIPLY $\emptyset V_{uc}$ x 0.55	$V_{as}$	MULTIPLY $\emptyset V_{us}$ x 0.50
Cyclic Loading	$V_{yc}$	MULTIPLY $\emptyset V_{uc}$ x 0.55	$V_{ys}$	MULTIPLY $\emptyset V_{us}$ x 0.50
Fire Resistance	$V_{Rk,c,fi,t}$	Refer to pages 238-257	$V_{Rk,s,fi,t}$	Refer to pages 238-257
Cracked Concrete/Tension Zone	$V_{Rd,c}^0$	Refer to pages 258-298	$V_{Rd,s}^0$	Refer to pages 258-298
Seismic	$V_{Rd,c,sis}^0$	Refer to pages 299-325	$V_{Rd,s,sis}^0$	Refer to pages 299-325

NOTE: Design Shear Capacity is the minimum of Concrete Shear and Steel Shear Capacities

**STEP 6 Combined loading and specification**

**Checkpoint 6**

Check

$N^* / \emptyset N_{ur} + V^* / \emptyset V_{ur} \leq 1.2$ ,

if not satisfied return to step 1

**Specify**

Ramset™ SpaTec™ Plus Anchor,  
(Anchor Size) ((Part Number)).  
Maximum fixed thickness to be (t) mm.

**Example**

Ramset™ SpaTec™ Plus Anchor,  
M12 (SP12120).  
Maximum fixed thickness to be 8 mm.  
To be installed in accordance with  
Ramset™ Technical Data Sheet.