

AVAILABLE IN AUSTRALIA ONLY (New Zealand refer to Epcon™ C6)

15.1 GENERAL INFORMATION

PERFORMANCE RELATED	INSTALLATION RELATED

Product

ChemSet™ Reo 502™ is an extra heavy duty pure epoxy anchoring adhesive.

Benefits, Advantages and Features

Greater productivity:

- Anchors in dry, damp, wet or flooded holes – no weather delays
- Fast 3 hour cure time (REO502J)
- Short embedment to achieve bar yield stress

Greater security:

- Exceeds steel strength up to **AISI 316** Stainless Steel at recommended embedments
- High loads in shallow holes in thin slabs

Versatile pure epoxy:

- Anchors in dry, damp, wet and flooded
- Oversized holes into 2.25x_d
- Anchors in carbide drilled and diamond cored holes
- For tropical and temperate climates
- Easy flow version for easy cold dispensing (REO502™EF)

Greater safety:

- Low odour
- Non-flammable for transport and storage
- VOC Compliant

Australian Made



Principal Applications

- Reinforcing bars
- Deformed bars
- Starter bars
- Anchoring structural steel to concrete
- Anchoring stadium seating

Installation temperature limits:

REO502™

- Substrate: 5°C to 40°C.
- Adhesive: 20°C to 40°C.

REO502™EF

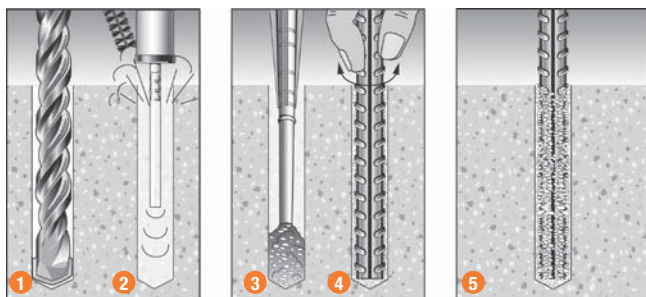
- Substrate: 5°C to 30°C.
- Adhesive: 5°C to 40°C.

Load should not be applied to anchor until the chemical has sufficiently cured as specified.

Service temperature limits:

-40°C to 80°C.

Installation



1. Drill recommended diameter and depth hole.
2. **Important:** Use **Ramset™** Dustless Drilling System to ensure holes are clean. Alternatively, clean dust and debris from hole with stiff wire or nylon brush and blower in the following sequence: blow x 4, brush x 3, blow x 4, brush x 3, blow x 4.
3. Insert mixing nozzle to bottom of hole. Fill hole to 3/4 the hole depth slowly, ensuring no air pockets form.
4. Insert rebar to bottom of hole while turning.
5. Allow ChemSet™ Reo 502™ to cure as per setting times.

Setting Times

Substrate Temperature	Reo 502™		Reo 502™EF	
	Working Time (mins)	Cure Time (hrs)	Working Time (mins)	Cure Time (hrs)
40°C	8.5	2	8.5	6
30°C	12	2.5		
25°C				
20°C	20	3	13	12
15°C	23	5	17	22
10°C	27	8	22	48
5°C			32	96
0°C				

Note: Cartridge temperature minimum 20°C.

Note: Cure time is extended in flooded conditions. Refer to Technical Data Sheet.

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Installation and performance details:
ChemSet™ Reo 502™ and Reinforcing Bar

Anchor Size, d_b (mm)	Drilled Hole diam., d_h (mm)	Anchor Effective Depth, h (mm)	Optimum dimensions*			Reduced Characteristic Capacity #				
			Edge* distance, e_c (mm)	Anchor spacing, a_c (mm)	Concrete substrate thickness, b_m (mm)	Gr 500 Rebar - Steel		Concrete		
						Tension, ΦN_{us} (kN)***	Shear, ΦV_{us} (kN)	Tension, ΦN_{uc} (kN)**		
								Concrete compressive strength, f'_c		
20 MPa	32 MPa	40 MPa								
10	14	90	40	60	120	31.4	21.4	14.8	18.7	20.9
12	16	110	50	70	140	45.2	30.8	21.7	27.4	30.7
16	20	125	65	100	160	80.4	54.8	32.9	41.6	46.5
20	25	150	80	120	190	125.6	85.7	49.3	62.4	69.7
		170						220	55.9	70.7
24	30	180	100	145	240	180.8	123.3	71.0	89.8	100.4
		210			270			82.8	104.8	117.2
25	30	180	100	150	240	196.4	133.9	74.0	93.6	104.6
		210			270			86.3	109.2	122.0
28	35	225	115	170	295	246.4	168.0	103.6	131.0	146.5
		270			340			124.3	157.2	175.7
32	40	240	130	195	320	321.6	219.3	126.2	159.7	178.5
		300			380			157.8	199.6	223.2
36	45	290	145	220	380	408.0	278.3	171.6	217.1	242.7
		330			420			195.3	247.0	276.2
40	50	320	160	240	420	504.0	343.7	210.4	266.1	297.6
		360			460			236.7	299.4	334.8

* Note: For shear loads acting towards an edge or where these optimum dimensions are not achievable, please use the simplified strength limit state design process to verify capacity.
 Note: Reduced characteristic ultimate concrete tensile capacity = ΦN_{uc} where $\Phi = 0.6$ and N_{uc} = Characteristic ultimate concrete tensile capacity. **For conversion to Working Load Limit MULTIPLY ΦN_{uc} x 0.55
 ***Note: Reduced characteristic ultimate steel tensile capacity = ΦN_{us} where $\Phi = 0.8$ and N_{us} = Characteristic ultimate steel tensile capacity. **For conversion to Working Load Limit MULTIPLY ΦN_{us} x 0.56**
 #Note: Design Tensile Capacity ΦN_{ur} = minimum of ΦN_{uc} and ΦN_{us}
WET HOLES: Multiply ΦN_{uc} x 0.7

15.2 DESCRIPTION AND PART NUMBERS

Description	Cartridge Size	Part No.
ChemSet™ Reo 502™	750 ml	Reo502J
ChemSet™ Reo 502EF™	750 ml	Reo502JEF

Effective depth, h (mm)

$h \geq 6 * d_h$

(To obtain full steel strength in shear)

Substrate thickness, b_m (mm)

$b_m = \text{greater of: } 1.25 * h, h + (2 * d_h)$

Drilled hole depth, h_1 (mm)

$h_1 = h$
 $h = \text{Effective depth}$

15.3 ENGINEERING PROPERTIES

Typical Engineering Properties of Grade 500 Reinforcing Bar

Rebar Size	10	12	16	20	24	25	28	32	36	40
Drilled Hole Dia, d_h (mm)	14	16	20	25	30	30	35	40	45	50
Stress Area, A_s (mm ²)	78.5	113	201	314	452	491	616	804	1020	1260
Yield Stress, f_{sy} (MPa)	500	500	500	500	500	500	500	500	500	500
Tensile Steel Yield Capacity N_{sy} (KN)	39.3	56.5	100.5	157.0	226.0	245.5	308.0	402.0	510.0	630.0

For further information refer to reinforcing bar manufacturer's published information and AS/NZS 4671:2001

Chemical Anchoring - Reinforcing Bar Anchorage

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STEP 1 Select anchor to be evaluated

Table 1a Indicative combined loading – interaction diagram

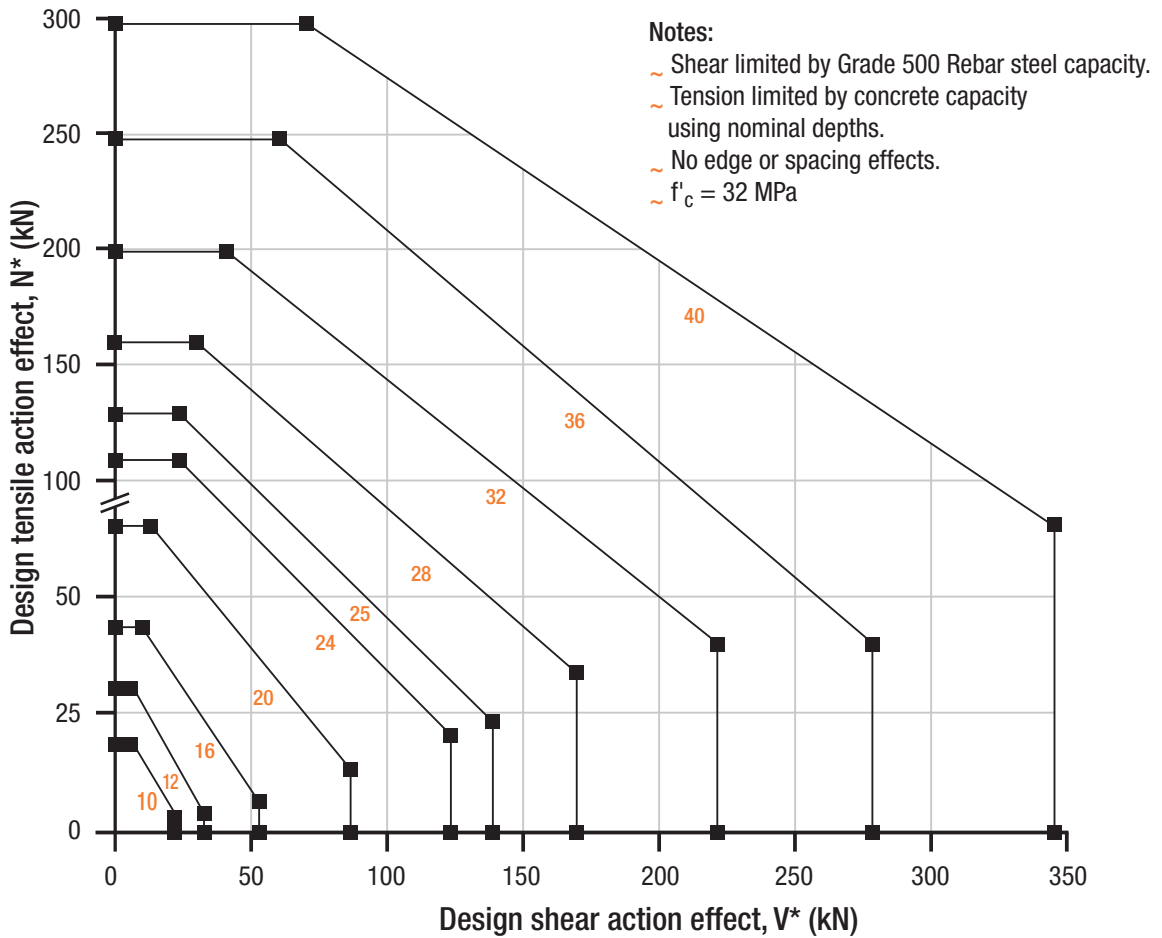


Table 1b Absolute minimum edge distance and anchor spacing values, e_m and a_m (mm)

Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
e_m, a_m	30	36	48	60	72	75	84	96	108	120

Step 1c Calculate anchor effective depth, h (mm)

Refer to nominal recommended effective depths, h , listed in installation and performance details table on page 112.

Effective depth, h (mm)

$h \geq 6 * d_h$

(To obtain full steel strength in shear)

Checkpoint 1 Anchor size determined, absolute minima compliance achieved, effective depth (h) calculated.

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Chemical Anchoring - Reinforcing Bar Anchorage

STEP 2 Verify concrete tensile capacity - per anchor

Table 2a Reduced characteristic ultimate concrete tensile capacity, ϕN_{uc} (kN), $\phi_c = 0.6$, $f'_c = 32$ MPa

Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
Drilled Hole Dia, d_h (mm)	14	16	20	25	30	30	35	40	45	50
Effective Depth, h (mm)										
85	17.7									
90	18.7									
95	19.8									
100	20.8	25.0								
105	21.8	26.2								
110	22.9	27.4								
115	23.9	28.7								
125	26.0	31.2	41.6							
140	29.1	34.9	46.6							
150	31.2	37.4	49.9	62.4						
170		42.4	56.6	70.7						
180		44.9	59.9	74.9	89.8	93.6				
210			69.9	87.3	104.8	109.2	122.3			
240			79.8	99.8	119.8	124.8	139.7	159.7		
270				112.3	134.7	140.4	157.2	179.6	202.1	
300				124.8	149.7	155.9	174.7	199.6	224.6	249.5
320				133.1	159.7	166.3	186.3	212.9	239.5	266.1
330					164.7	171.5	192.1	219.6	247.0	274.5
360					179.6	187.1	209.6	239.5	269.5	299.4
420						218.3	244.5	279.5	314.4	349.3
460								306.1	344.3	382.6
500								332.7	374.3	415.9
550									411.7	457.4
600										499.0
625										519.8

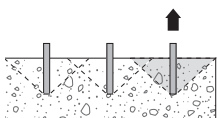
Bold values are at ChemSet™ Anchor Stud nominal depths.

Note: Effective depth, h must be $\geq 6 \times$ drilled hole diameter, d_h for anchor to achieve tabled shear capacities. Wet holes: Multiply $\phi N_{uc} * 0.7$.

Table 2b Concrete compressive strength effect, tension, X_{nc}

f'_c (MPa)	20	25	32	40	50
X_{nc}	0.79	0.88	1.00	1.12	1.25

Table 2c Edge distance effect, tension, X_{ne}

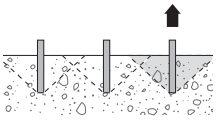


Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
Edge Distance, e (mm)										
30	0.83									
35	0.91	0.81								
40	1.00	0.88								
50		1.00	0.85							
65			1.00	0.87						
80				1.00	0.88	0.86				
95					1.00	0.97	0.89	0.82		
100						1.00	0.93	0.85		
110							1.00	0.90	0.83	
130								1.00	0.93	0.87
145									1.00	0.93
160										1.00

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Table 2d Anchor spacing effect, end of a row, tension, X_{nae}

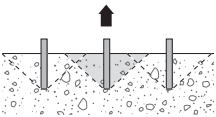
For single anchor design, $X_{nae} = 1.0$



Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
Anchor Spacing, a (mm)										
30	0.75									
35	0.79	0.74								
40	0.83	0.78								
50	0.92	0.85	0.76							
60	1.00	0.92	0.81	0.75						
75		1.00	0.89	0.81	0.76	0.75				
95			1.00	0.90	0.83	0.82	0.78			
120				1.00	0.92	0.90	0.86	0.81	0.78	0.75
140					1.00	0.97	0.92	0.86	0.82	0.79
150						1.00	0.95	0.89	0.85	0.81
170							1.00	0.94	0.89	0.85
195								1.00	0.95	0.91
220									1.00	0.96
240										1.00

Table 2e Anchor spacing effect, internal to a row, tension, X_{nai}

For single anchor design, $X_{nai} = 1.0$



Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
Anchor Spacing, a (mm)										
30	0.50									
35	0.58	0.49								
40	0.67	0.56								
50	0.83	0.69	0.52							
60	1.00	0.83	0.63	0.50						
75		1.00	0.78	0.63	0.52	0.50				
95			1.00	0.79	0.66	0.63	0.57	0.49		
120				1.00	0.83	0.80	0.71	0.63	0.56	0.50
150					1.00	1.00	0.89	0.78	0.69	0.63
170							1.00	0.89	0.79	0.71
195								1.00	0.90	0.81
215									1.00	0.90
240										1.00

Checkpoint 2 Design reduced ultimate concrete tensile capacity, ϕN_{urc}

$$\phi N_{urc} = \phi N_{uc} * X_{nc} * X_{ne} * (X_{nae} \text{ or } X_{nai})$$

STEP 3 Verify anchor tensile capacity - per anchor

Table 3a Reduced characteristic ultimate steel tensile capacity, ϕN_{us} (kN), $\phi_n = 0.8$

Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
e_m, a_m	31.4	45.2	80.4	125.6	180.8	196.4	246.4	321.6	408.8	504.0

Step 3b Reduced characteristic ultimate bolt steel tensile capacity, ϕN_{tr} (kN)

Not appropriate for this product.

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Checkpoint **3**

Design reduced ultimate tensile capacity, $\emptyset N_{ur}$

$$\emptyset N_{ur} = \text{minimum of } \emptyset N_{urc}, \emptyset N_{us}$$

$$\text{Check } N^* / \emptyset N_{ur} \leq 1,$$

if not satisfied return to step 1

Tensile performance conversion table

Performance Required	Concrete Tensile Performance		Steel Tensile Performance	
	Notation	Concrete Tension Capacity	Notation	Carbon Steel Tension Capacity
Strength Limit State	$\emptyset N_{urc}$	MULTIPLY $\emptyset N_{urc} \times 1.00$	$\emptyset N_{us}$	MULTIPLY $\emptyset N_{us} \times 1.00$
Working Load Limit	N_{ac}	MULTIPLY $\emptyset N_{urc} \times 0.55$	N_{as}	MULTIPLY $\emptyset N_{us} \times 0.56$
Cyclic Loading	N_{yc}	MULTIPLY $\emptyset N_{urc} \times 0.55$	N_{ys}	MULTIPLY $\emptyset N_{us} \times 0.56$
Fire Resistance	$N_{Rk,c,fi,t}$	Refer to pages 238-257	$N_{Rk,s,fi,t}$	Refer to pages 238-257
Cracked Concrete/ Tension Zone	$N_{Rd,p}^0$	Refer to pages 258-298	$N_{Rd,s}$	Refer to pages 258-298
Seismic	$N_{Rd,p,sis}^0$	Refer to pages 299-325	$N_{Rd,s,sis}$	Refer to pages 299-325

NOTE: Design Tensile Capacity is the minimum of Concrete Tension and Steel Tension Capacities

STEP 4 Verify concrete shear capacity - per anchor

Table 4a Reduced characteristic ultimate concrete edge shear capacity, $\emptyset V_{uc}$ (kN), $\emptyset_q = 0.6$, $f'_c = 32$ MPa

Anchor size, d_h	10	12	16	20	24	25	28	32	36	40
Edge Distance, e (mm)										
30	2.5									
35	3.2									
40	3.9	4.2								
50	5.5	5.9	6.5							
60	7.2	7.7	8.6	9.6						
75	10.1	10.8	12.0	13.4	14.7	14.7				
95	14.3	15.3	17.1	19.2	21.0	21.0	22.7			
120	20.4	21.8	24.3	27.2	29.8	29.8	32.2	34.4	36.5	38.5
200	43.8	46.8	52.4	58.6	64.1	64.1	69.3	74.1	78.6	82.8
300	80.5	86.1	96.2	107.6	117.8	117.8	127.3	136.1	144.3	152.1
400	123.9	132.5	148.1	165.6	181.4	181.4	196.0	209.5	222.2	234.2
500	173.2	185.2	207.0	231.5	253.6	253.6	273.9	292.8	310.6	327.4
600	227.7	243.4	272.2	304.3	333.3	333.3	360.0	384.9	408.2	430.3

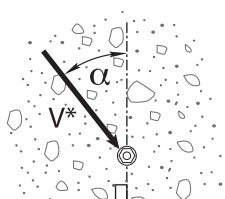
Note: Effective depth, h must be $\geq 6 \times$ drilled hole diameter, d_h for anchor to achieve tabled shear capacities.

Table 4b Concrete compressive strength effect, concrete edge shear, X_{vc}

f'_c (MPa)	20	25	32	40	50
X_{vc}	0.79	0.88	1.00	1.12	1.25

Table 4c Load direction effect, concrete edge shear, X_{vd}

Angle, α°	0	10	20	30	40	50	60	70	80	90 - 180
X_{vd}	1.00	1.04	1.16	1.32	1.50	1.66	1.80	1.91	1.98	2.00



Load direction effect, conc. edge shear, X_{vd}

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Table 4d Anchor spacing effect, concrete edge shear, X_{va}

Note: For single anchor designs, $X_{va} = 1.0$

Edge distance, e (mm)	25	30	35	50	60	75	125	200	300	400	500	600
Anchor spacing, a (mm)												
25	0.70	0.67	0.64	0.60	0.58	0.57	0.54					
30	0.74	0.70	0.67	0.62	0.60	0.58	0.55	0.53				
35	0.78	0.73	0.70	0.64	0.62	0.59	0.56	0.54	0.52			
50	0.90	0.83	0.79	0.70	0.67	0.63	0.58	0.55	0.53	0.53		
60	0.98	0.90	0.84	0.74	0.70	0.66	0.60	0.56	0.54	0.53	0.52	
75	1.00	1.00	0.93	0.80	0.75	0.70	0.62	0.58	0.55	0.54	0.53	0.53
150			1.00	1.00	1.00	0.90	0.74	0.65	0.60	0.58	0.56	0.55
200						1.00	0.82	0.70	0.63	0.60	0.58	0.57
300							0.98	0.80	0.70	0.65	0.62	0.60
400							1.00	0.90	0.77	0.70	0.66	0.63
500								1.00	0.83	0.75	0.70	0.67
625									0.92	0.81	0.75	0.71
750									1.00	0.88	0.80	0.75
875										0.94	0.85	0.79
1000										1.00	0.90	0.83
1250											1.00	0.92
1500												1.00

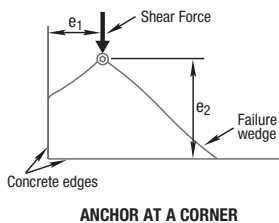
Table 4e Multiple anchors effect, concrete edge shear, X_{vn}

Note: For single anchor designs, $X_{vn} = 1.0$

Anchor spacing / Edge distance, a / e	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00	2.25	2.50
Number of anchors, n												
2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
3	0.72	0.76	0.80	0.83	0.86	0.88	0.91	0.93	0.95	0.96	0.98	1.00
4	0.57	0.64	0.69	0.74	0.79	0.82	0.86	0.89	0.92	0.94	0.97	1.00
5	0.49	0.57	0.63	0.69	0.74	0.79	0.83	0.87	0.90	0.93	0.97	1.00
6	0.43	0.52	0.59	0.66	0.71	0.77	0.81	0.85	0.89	0.93	0.96	1.00
7	0.39	0.48	0.56	0.63	0.69	0.75	0.80	0.84	0.88	0.92	0.96	1.00
8	0.36	0.46	0.54	0.61	0.68	0.74	0.79	0.84	0.88	0.92	0.96	1.00
9	0.34	0.44	0.52	0.60	0.67	0.73	0.78	0.83	0.87	0.91	0.96	1.00
10	0.32	0.42	0.51	0.59	0.66	0.72	0.77	0.82	0.87	0.91	0.96	1.00
15	0.26	0.37	0.47	0.55	0.63	0.70	0.76	0.81	0.86	0.90	0.95	1.00
20	0.23	0.35	0.45	0.54	0.61	0.68	0.75	0.80	0.85	0.90	0.95	1.00

Table 4f Anchor at a corner effect, concrete edge shear, X_{vs}

Note: For $e_1/e_2 > 1.25$, $X_{vs} = 1.0$



Edge distance, e_2 (mm)	25	30	35	50	60	75	125	200	300	400	600	900
Edge distance, e_1 (mm)												
25	0.86	0.77	0.70	0.58	0.53	0.49	0.41	0.37	0.35	0.34	0.32	0.32
30	0.97	0.86	0.78	0.64	0.58	0.52	0.43	0.38	0.36	0.34	0.33	0.32
35	1.00	0.95	0.86	0.69	0.63	0.56	0.46	0.40	0.37	0.35	0.33	0.32
50	1.00	1.00	1.00	0.86	0.77	0.67	0.52	0.44	0.39	0.37	0.35	0.33
60	1.00	1.00	1.00	0.97	0.86	0.75	0.57	0.47	0.41	0.38	0.36	0.34
75	1.00	1.00	1.00	1.00	1.00	0.86	0.64	0.51	0.44	0.41	0.37	0.35
125	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.65	0.53	0.48	0.42	0.38
200	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67	0.58	0.49	0.42
300	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.72	0.58	0.49
400	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67	0.55
500	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.77	0.61
600	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.67
900	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.86

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Checkpoint 4

Design reduced ultimate concrete edge shear capacity, $\emptyset V_{urc}$

$$\emptyset V_{urc} = \emptyset V_{uc} * X_{vc} * X_{vd} * X_{va} * X_{vn} * X_{vs}$$

STEP 5

Verify anchor shear capacity - per anchor

Table 5a Reduced characteristic ultimate steel shear capacity, $\emptyset V_{us}$ (kN), $\emptyset_v = 0.8$

Anchor size, d_b	10	12	16	20	24	25	28	32	36	40
Grade 500 Rebar	21.4	30.8	54.8	85.7	123.3	133.9	168.0	219.3	278.3	343.7

Step 5b Reduced characteristic ultimate bolt steel shear capacity, $\emptyset V_{sf}$ (kN)

Not appropriate for this product.

Checkpoint 5

Design reduced ultimate shear capacity, $\emptyset V_{ur}$

$$\emptyset V_{ur} = \text{minimum of } \emptyset V_{urc}, \emptyset V_{us}$$

$$\text{Check } V^* / \emptyset V_{ur} \leq 1,$$

if not satisfied return to step 1

Shear performance conversion table

Performance Required	Concrete Shear Performance		Steel Shear Performance	
	Notation	Concrete Shear Capacity	Notation	Carbon Steel Shear Capacity
Strength Limit State	$\emptyset V_{uc}$	MULTIPLY $\emptyset V_{uc}$ x 1.00	$\emptyset V_{us}$	MULTIPLY $\emptyset V_{us}$ x 1.00
Working Load Limit	V_{ac}	MULTIPLY $\emptyset V_{uc}$ x 0.55	V_{as}	MULTIPLY $\emptyset V_{us}$ x 0.45
Cyclic Loading	V_{yc}	MULTIPLY $\emptyset V_{uc}$ x 0.55	V_{ys}	MULTIPLY $\emptyset V_{us}$ x 0.45
Fire Resistance	$V_{Rk,c,fi,t}$	Refer to pages 238-257	$V_{Rk,s,fi,t}$	Refer to pages 238-257
Cracked Concrete/Tension Zone	$V_{Rd,c}^0$	Refer to pages 258-298	$V_{Rd,s}^0$	Refer to pages 258-298
Seismic	$V_{Rd,c,sis}^0$	Refer to pages 299-325	$V_{Rd,s,sis}^0$	Refer to pages 299-325

NOTE: Design Shear Capacity is the minimum of Concrete Shear and Steel Shear Capacities

STEP 6

Combined loading and specification

Checkpoint 6

Check

$$N^* / \emptyset N_{ur} + V^* / \emptyset V_{ur} \leq 1.2,$$

if not satisfied return to step 1

Specify – Reinforcing Bar Anchorage

Ramset™ ChemSet™ Reo 502™ with
(Anchor Size) grade 500 Rebar.
Drilled hole depth to be (h) mm.

Example

Ramset™ ChemSet™ Reo 502™ with
N20 grade 500 Rebar
Drilled hole depth to be 160 mm.
To be installed in accordance with
Ramset™ Technical Data Sheet.